



# Euler-Bernoulli Type Beam Theory for Elastic Material with Nonlinear Response in the Small Strain Range

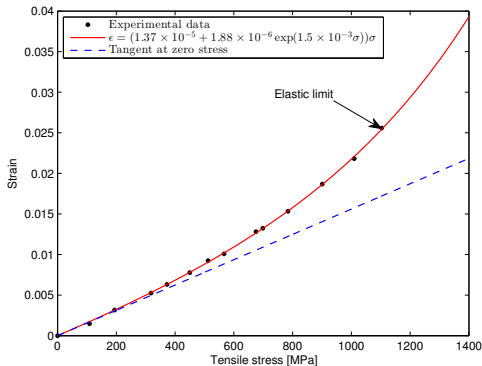
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Workshop on MOdel REduction

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# Materials with Nonlinear Response

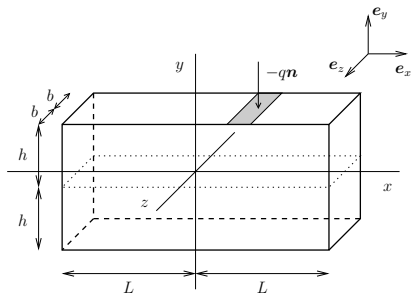


**Figure :** Strain-stress relation for a Gum metal alloy (Ti-12Ta-9Nb-3V-6Zr-1.5O).



Saito, T., et al. (2003). Multifunctional Alloys Obtained via a Dislocation-Free Plastic Deformation Mechanism. *Science* 300 (5618), 464–467.

# Euler-Bernoulli Type Beam Theory



**Figure :** Problem geometry.

# Euler-Bernoulli Type Beam Theory

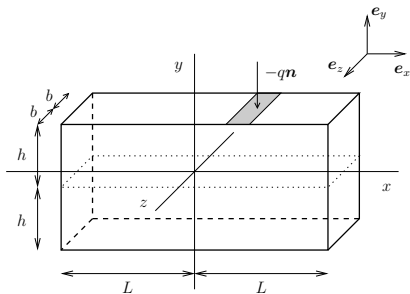


Figure : Problem geometry.

Stress field

$$\mathbb{T} = \begin{bmatrix} T_{\hat{x}\hat{x}} & T_{\hat{x}\hat{y}} & 0 \\ T_{\hat{x}\hat{y}} & T_{\hat{y}\hat{y}} & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

# Euler-Bernoulli Type Beam Theory

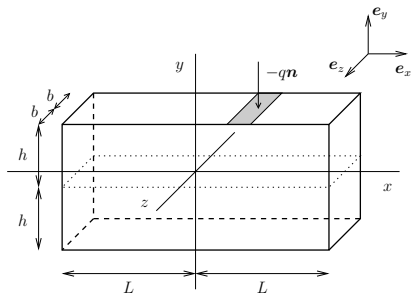


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## Stress field

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## Governing equations

$$\frac{dV}{dx} - q = 0$$

$$\frac{dM}{dx} - V = 0$$

$$V =_{\text{def}} \int_{y=-h}^h T_{\hat{x}\hat{y}} dy$$

$$M =_{\text{def}} \int_{y=-h}^h y T_{\hat{x}\hat{x}} dy$$

# Euler-Bernoulli Type Beam Theory

Small strain tensor

$$\boldsymbol{\epsilon} =_{\text{def}} \frac{1}{2} (\nabla \mathbf{u} + (\nabla \mathbf{u})^T) \approx \begin{bmatrix} -y \frac{d^2 w}{dx^2} & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

# Euler-Bernoulli Type Beam Theory

Small strain tensor

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- Two kinds of **lateral loads** acting on the beam
  - Uniform load

$$q(x) = q^* > 0$$

- Concentrated load

$$q(x) = \tilde{q} \delta(x), \quad \tilde{q} > 0$$

- **Linearized elasticity**

$$\mathbb{T} = \lambda (\text{Tr } \epsilon) \mathbb{1} + 2\mu \epsilon$$

- Material with **nonlinear response**

$$\epsilon = \lambda_1 (\text{Tr } \mathbb{T}) \mathbb{1} + 2\lambda_2 e^{\eta \text{Tr } \mathbb{T}} \mathbb{T}$$

$$\epsilon = \lambda_1 (\text{Tr } \mathbb{T}) \mathbb{1} + \lambda_2 \left(1 + \alpha |\mathbb{T}|^2\right)^n \mathbb{T}$$

$$\epsilon = \gamma_1 (\text{Tr } \mathbb{T}) \mathbb{1} + \sinh [(\text{Tr } \mathbb{T})^{\gamma_2} / \gamma_3] \mathbb{1} + \gamma_4 \mathbb{T}$$



Rajagopal, K. R. (2014). On the nonlinear elastic response of bodies in the small strain range. *Acta Mech.* 225 (6), 1545–1553.



Grasley, Z., R. El-Helou, M. D'Ambrosia, D. Mokarem, C. Moen, and K. R. Rajagopal (2015). Model of infinitesimal nonlinear elastic response of concrete subjected to uniaxial compression. *J. Eng. Mech.* 141 (7), 04015008.



- System of **governing equations**:

$$\frac{d^2 M}{dx^2} = q$$

$$\varepsilon_{\hat{x}\hat{x}} = -y \frac{d^2 w}{dx^2}$$

$$\frac{d^2}{dx^2} \left( \int_{y=-h}^h y T_{\hat{x}\hat{x}} dy \right) = q$$

$$y \frac{d^2 w}{dx^2} = - \left( \lambda_1 + 2\lambda_2 e^{\eta T_{\hat{x}\hat{x}}} \right) T_{\hat{x}\hat{x}}$$

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- Fixed ends** boundary conditions

$$w|_{x=\pm L} = \frac{dw}{dx} \Big|_{x=\pm L} = 0$$

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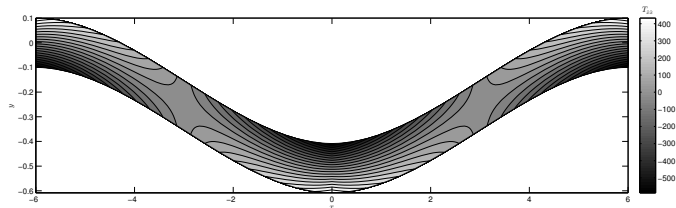
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- Spectral collocation method**

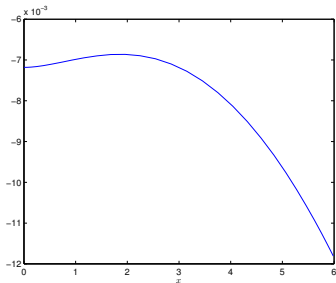
- Gauss-Lobatto-Chebyshev points:
- Spectral differentiation matrices
- Clenshaw-Curtis quadrature

$$x_i = \cos \left( \frac{(i-1)\pi}{N-1} \right), \quad i = 1, \dots, N$$

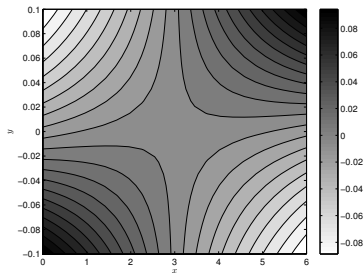


**Figure :** Stress field of a beam with nonlinear response under a concentrated load at  $x = 0$ ,  $L = 6$  m,  $h = 0.1$  m,  $\lambda_1 = 1.37 \times 10^{-5}$  MPa $^{-1}$ ,  $\lambda_2 = 9.38 \times 10^{-7}$  MPa $^{-1}$ ,  $\eta = 1.5 \times 10^{-3}$  MPa $^{-1}$ ,  $\tilde{q} = 2.4$  MPa.

# Numerical Solution – Results



(a) Relative difference of the deflection  $(w^{\text{LIN}} - w) / w^{\text{LIN}}$ .



(b) Relative difference of the stress field  $(T_{\hat{x}\hat{x}}^{\text{LIN}} - T_{\hat{x}\hat{x}}) / T_{\hat{x}\hat{x}}^{\text{LIN}}$ .

**Figure :** Comparison of the deflection and the stress field of the linearized elastic beam and a beam with nonlinear response under a concentrated load at  $x = 0$ ,  $L = 6$  m,  $h = 0.1$  m,  $\lambda_1 = 1.37 \times 10^{-5} \text{ MPa}^{-1}$ ,  $\lambda_2 = 9.38 \times 10^{-7} \text{ MPa}^{-1}$ ,  $\eta = 1.5 \times 10^{-3} \text{ MPa}^{-1}$ ,  $\tilde{q} = 2.4 \text{ MPa}$ .

**Thank You for Your Attention**